

CE243A - DESIGN AND RESPONSE OF RC STRUCTURAL SYSTEMS

Problem Set #5: Frame Design and Detailing

Due: 17 Nov. 2004

Analysis results for gravity and earthquake loading of an interior column and beam of a building frame are given below. Use $f'_c = 4000$ psi and $f_y = 60$ ksi.

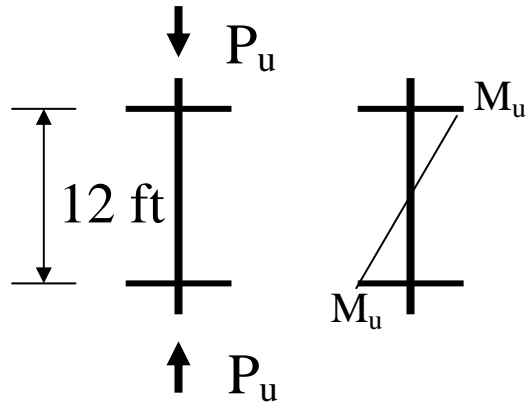
- 5.1 Determine the moment and shear envelopes for the ACI 318-02 load combinations (S9.2). Assume all loads given below are unfactored. For gravity loads, use values from ACI 318 S8.3 for a typical interior span (e.g., see MacGregor, Appendix A or similar reference).
- 5.2 Design an economical beam section for the given loads. Assume a 10 inch thick slab and that #5 @ 9 in. on center are used for negative slab reinforcement within the column strip. Assume slab bottom reinforcement is not continuous at the beam ends. Check flexural and shear strength requirements.
- 5.3 Verify that development length requirements are satisfied for the beam using appropriate provisions in Chapter 12 and 21 (Chapter 21 of ACI 318-02, Sections 21.1 through 21.3). Indicate results with a sketch (with cross section and elevation views) of the beam with dimensions and notes. For anchorage, plot the $\phi M_n(x) > M_u$ to verify that your beam has sufficient moment strength to avoid yielding along the beam span. Do not terminate (cut) any flexural reinforcement within a tension region (extend all reinforcement beyond inflection points).
- 5.4 Design an economical column section for the given loading using ACI 318-02 requirements. Check flexure and shear requirements, as well as requirements for confinement (Chapter 21, Sections 21.1 through 21.4). Ensure that your column satisfies the strong-column, weak-beam requirements in ACI 318-02 Section 21.4.2.2. Indicate results on a sketch (with cross section and elevation views) of the column with dimensions and notes.

Beam Gravity Loads: $w_{DL} = 2$ k/ft (unfactored)
 $w_{LL} = 1$ k/ft (unfactored)

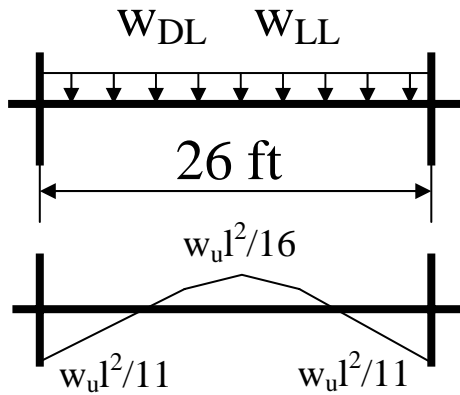
Column Gravity Loads: $P_{DL} = 600$ k (unfactored)
 $P_{LL} = 200$ k (unfactored)

Earthquake Beam Moments: $M_{eq} = 500$ ft-kips (unfactored)

Beam & Column geometry and loading (2D frame):

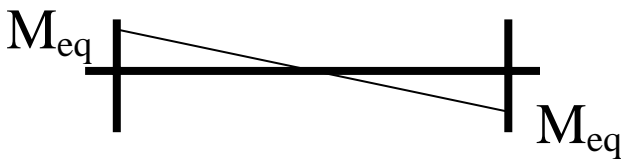


Column loading



Beam loading

Gravity moment coeff.
(ACI S8.3, interior span)



Earthquake moments
(+/-) unfactored